## Chapter 3

## Weight-Volume Relationships



## Useful Formulas

$$
\begin{gathered}
\checkmark \mathrm{V}=\mathrm{V}_{\mathrm{s}}+\mathrm{V}_{\mathrm{v}} \\
\mathrm{~V}_{\mathrm{v}}=\mathrm{V}_{\mathrm{w}}+\mathrm{V}_{\mathrm{a}} \\
\checkmark \mathrm{~W}=\mathrm{W}_{\mathrm{s}}+\mathrm{W}_{\mathrm{w}} \\
\checkmark \text { Void Ratio }
\end{gathered}
$$

(is defined as the ratio of the volume of voids to the volume of solids.)

$$
e=\frac{V v}{V s}
$$

$\checkmark$ Porosity
(is defined as the ratio of the volume of voids to the total volume)
$n=\frac{V v}{V}$
$\checkmark$ Degree Of Saturation
(is defined as the ratio of the volume of water to the volume of voids,)
$\mathrm{S}=\frac{\mathrm{Vw}}{\mathrm{V} \mathrm{v}}$

- $\mathrm{e}=\frac{\mathrm{Vv}}{\mathrm{Vs}}=\frac{\mathrm{Vv}}{\mathrm{V}-\mathrm{Vv}}=\frac{\frac{\mathrm{Vv}}{\mathrm{v}}}{\frac{\mathrm{V}}{\mathrm{V}}-\frac{\mathrm{Vv}}{\mathrm{V}}}=\frac{\mathrm{n}}{1-\mathrm{n}}$
- $\mathrm{n}=\frac{\mathrm{Vv}}{\mathrm{V}}=\frac{\mathrm{Vv}}{\mathrm{Vs}+\mathrm{Vv}_{\mathrm{v}}}=\frac{\frac{\mathrm{Vv}}{\mathrm{V}_{s}}}{\frac{\mathrm{Vs}}{\mathrm{Vs}} \frac{\mathrm{VV}}{\mathrm{Vs}}}=\frac{\mathrm{e}}{1+\mathrm{e}}$


## $\checkmark$ Moisture Content

(is also referred to as water content and is defined as the ratio of the weight of water to the weight of solids in a given volume of soil)
$\mathrm{w} \%=\frac{\mathrm{Ww}}{\mathrm{Ws}}$
$\checkmark$ Unit Weight
(is the weight of soil per unit volume)
$\mathrm{V}=\frac{\mathrm{W}}{\mathrm{V}}=\frac{\mathrm{Ww}+\mathrm{Ws}}{\mathrm{V}}=\frac{\mathrm{Ws}\left(1+\frac{\mathrm{WW}}{\mathrm{Ws}}\right)}{\mathrm{V}}=\frac{\mathrm{Ws}(1+\mathrm{w})}{\mathrm{V}}$
$\checkmark$ Dry Unit Weight
$\gamma_{d}=\frac{W s}{V}$
$\gamma=\gamma_{d}(1+w)$
$\checkmark$ Density
$\rho=\frac{\mathrm{M}}{\mathrm{V}} \mathrm{kg} / \mathrm{m} 3$
$\checkmark$ Dry Density
$\rho_{d}=\frac{M s}{V}$
$\checkmark$ Specific gravity
$\gamma=\frac{G S * X w(1+w)}{1+e}$
$\gamma_{d}=\frac{G S * V w}{1+e}$
$\checkmark$ S.e=GS.w
$\checkmark$ For Saturation

- $\gamma s a t=\frac{\gamma w(G s+e)}{1+e}$
- $\mathrm{S}=100 \%$, so $\mathrm{e}=\mathrm{Gs} . \mathrm{w}$
$\checkmark$ Relative density, $\mathrm{Dr}=\frac{\mathrm{emax}-\mathrm{e}}{\mathrm{emax}-\mathrm{emin}}$

Or $\operatorname{Dr}=\frac{\rho d-\rho d(\min )}{\rho d(\max )-\rho \mathrm{d}(\min )} * \frac{\rho \mathrm{~d}(\text { max })}{\rho \mathrm{d}}$
where
$\gamma_{d(\min )}$ : dry unit weight in the loosest condition (at a void ratio of emax).
$\gamma_{d}$ : in situ dry unit weight (at a void ratio of e).
$\gamma_{d(\max )}$ : dry unit weight in the densest condition (at a void ratio of emin).

### 3.4 A 0.4-m3 moist soil sample has the following:

- Moist mass $=711.2 \mathrm{~kg}$
- Dry mass = 623.9 kg
- Specific gravity of soil solids $=2.68$


## Estimate:

a. Moisture content
b. Moist density
c. Dry density
d. Void ratio
e. Porosity

Solution
a) $w \%=\frac{W w}{W s}$
$\mathrm{Ww}=\mathrm{W}-\mathrm{Ws}=711.2-623.9=87.3 \mathrm{~g}$

$$
\mathrm{w} \%=\frac{87.3}{623.9}=13.99 \%
$$

b) $\rho=\frac{\mathrm{M}}{\mathrm{V}}=\frac{711.2}{0.4}=1778 \mathrm{~kg} / \mathrm{m} 3$

$$
\rho_{\mathrm{d}}=\frac{\mathrm{Ms}}{\mathrm{~V}}=\frac{623.9}{.4}=1559.75 \mathrm{~kg} / \mathrm{m} 3
$$

c) $e=$ ??
$\rho d=\frac{G S * \rho w}{1+e}$
$1559.75=\frac{2.68 * 1000}{1+\mathrm{e}}$
Solve for $\mathrm{e}=0.7182$
d) $n=\frac{e}{1+e}=\frac{0.7182}{1+.7182}=0.418$
3.7 The saturated unit weight of a soil is 19.8 $\mathrm{kN} / \mathrm{m} 3$. The moisture content of the soil is
$\mathbf{1 7 . 1 \%}$. Determine the following:
a. Dry unit weight
b. Specific gravity of soil solids
c. Void ratio

Solution
$\gamma_{\text {sat. }}=19.8 \mathrm{KN} / \mathrm{m} 3, \mathrm{w} \%=17.1 \%$
a) $\gamma_{d}=$ ??

Ysat. $=\gamma \mathrm{d}(1+\mathrm{w})$
$\gamma \mathrm{d}=\frac{19.8}{1+0.171}=16.9 \mathrm{KN} / \mathrm{m} 3$
b) \& c) Gs=??, e=??
S.e=GS.w

1*e=Gs*0.171
$\mathrm{Yd}=\frac{\mathrm{GS} * \mathrm{Vw}}{1+\mathrm{e}}$
$16.9=\frac{\mathrm{GS} * 9.81}{1+\mathrm{e}}$
$16.9 \mathrm{e}+16.9=9.81 \mathrm{Gs}$
Solve eq. 1 and 2 for e and Gs

$$
\begin{align*}
\mathrm{e} & =0.4176  \tag{2}\\
\mathrm{Gs} & =2.442
\end{align*}
$$

3.22 For a given sandy soil, the maximum and minimum dry unit weights are $108 \mathrm{lb} / \mathrm{ft} 3$ and 92 $\mathbf{l b} / \mathrm{ft} 3$, respectively. Given $\mathrm{Gs}=\mathbf{2 . 6 5}$, determine the moist unit weight of this soil when the relative density is $60 \%$ and the moisture content is $\mathbf{8 \%}$.

$$
\begin{aligned}
& \text { Solution } \\
& \gamma_{\mathrm{d}(\max )}=108 \mathrm{Ib} / \mathrm{ft} 3, \mathrm{Zd}(\mathrm{~min})=92 \mathrm{Ib} / \mathrm{ft} 3 \text {, } \\
& \text { Gs=2.65, } \gamma=? ?, \mathrm{Dr}=60 \% \text {, w } \%=8 \% \\
& \mathrm{Dr}=\frac{\rho \mathrm{d}-\rho \mathrm{d}(\min )}{\rho \mathrm{d}(\max )-\rho \mathrm{d}(\min )} * \frac{\rho \mathrm{~d}(\max )}{\rho \mathrm{d}} \\
& 0.6=\frac{\gamma d-92}{108-92} * \frac{108}{\gamma d} \\
& \text { } \mathrm{\gamma d}=100.975 \mathrm{Ib} / \mathrm{ft} 3 \\
& \gamma d=\frac{G S * V w}{1+e} \\
& 100.975=\frac{2.65 * 62.4}{1+e} \\
& e=0.637 \\
& \mathrm{X}=\frac{\mathrm{GS} * \mathrm{Xw}(1+\mathrm{w})}{1+\mathrm{e}}=\frac{2.65 * 62.4(1+.08)}{1+.637} \\
& =109.095 \mathrm{Ib} / \mathrm{ft} 3
\end{aligned}
$$

### 3.24 A loose, uncompacted sand fill 6 ft in depth

 has a relative density of $\mathbf{4 0 \%}$. Laboratory tests indicated that the minimum and maximum void ratios of the sand are 0.46 and 0.90 , respectively. The specific gravity of solids of the sand is 2.65 .a. What is the dry unit weight of the sand?
b. If the sand is compacted to a relative density of $75 \%$, what is the decrease in thickness of the 6-ft fill?

Solution
Depth=6 ft, $\mathrm{Dr}=40 \%$, emax $=0.9$, emin $=0.46$, Gs=2.65
$\mathrm{dd}=$ ??
$\gamma \mathrm{d}=\frac{\mathrm{GS} * \mathrm{Xw}}{1+\mathrm{e}} \quad \ldots \mathrm{e}=$ ?
$\mathrm{Dr}=\frac{\mathrm{emax}-\mathrm{e}}{\text { emax-emin }}=\frac{0.9-\mathrm{e}}{0.9-.46}=0.4$ $\mathrm{e}=0.724$

$$
\mathrm{\gamma d}=\frac{2.65 * 62.4}{1+0.724}=95.916 \mathrm{Ib} / \mathrm{ft} 3
$$

decrease in thickness=??
$\mathrm{Yd}=95.916=\frac{\mathrm{Ws}}{\mathrm{V}}=\frac{\mathrm{Ws}}{\text { area } * 6}$
Ws=area*6*95.916.
After compaction:
$\operatorname{Dr}=0.75, \mathrm{Dr}=\frac{\mathrm{emax}-\mathrm{e}}{\mathrm{emax}-\mathrm{emin}}=\frac{0.9-\mathrm{e}}{0.9-0.46}=0.75$
e=0.57
$\mathrm{Xd}=\frac{\mathrm{GS} * \mathrm{Xw}}{1+\mathrm{e}}=\frac{2.65 * 62.4}{1+0.57}=105.32 \mathrm{Ib} / \mathrm{ft} 3$
$\gamma \mathrm{d}=105.32=\frac{\mathrm{Ws}}{\mathrm{V}}=\frac{\mathrm{Ws}}{\text { area } * \text { thick }}$.
Ws=105.32*area*thick.....(2)

Solve 1 and 2
Thick. $=5.4643$
decrease in thickness $=6-5.4643=0.5357 \mathrm{ft}$

## Q2( Exam 2011)

An undisturbed sample of clayey soil is found to have a wet weight of 285 N , a dry weight of 250 N , and a total Volume of $14^{*} 10^{3} \mathrm{~cm} 3$.If the specific gravity of soil solid is 2.7 , determine the water content, void ratio, and degree of saturation.

$$
\begin{aligned}
& \text { Solution } \\
& \mathrm{W}=0.285 \mathrm{KN}, \mathrm{Ws}=0.25 \mathrm{KN}, \\
& \mathrm{~V}=14^{*} 10^{3 *} 10^{-6}=14^{*} 10^{-3} \mathrm{~m}^{3} \\
& \mathrm{~W} \%=\frac{\mathrm{Ww}}{\mathrm{Ws}}=\frac{0.285-0.25}{0.25}=0.14 \\
& \mathrm{e}=? ? \quad \mathrm{~V}=\frac{\mathrm{GS} * \mathrm{Vw}}{1+\mathrm{e}}=\frac{\mathrm{Ws}}{\mathrm{~V}}= \\
& \frac{0.25}{14 *(10)^{\wedge}-3}=17.857 \mathrm{Kn} / \mathrm{m} 3 \\
& 17.857=\frac{2.7 * 9.81}{1+\mathrm{e}} \\
& \mathrm{e}=0.483 \\
& \mathrm{~S}=? ? \\
& \mathrm{~S} . \mathrm{e}=\mathrm{GS} . \mathrm{W} \\
& \mathrm{~S}=\frac{2.7 * 0.14}{0.483}=0.782
\end{aligned}
$$

Q1 (Exam 2012)
A soil sample has a void ratio of 0.72 , moisture content $=\mathbf{1 2 \%}$ and $\mathbf{G s}=\mathbf{2 . 7 2}$ determine the following:
a)Dry Unit Weight, moist unit weight ( $\mathrm{KN} / \mathrm{m} 3$ )
b) Weight of water in $\mathrm{KN} / \mathrm{m} 3$ to be added for $\mathbf{8 0 \%}$ degree of saturation.
c)Is it possible to reach a water content of $\mathbf{3 0 \%}$ without change the present void ratio.

Solution
a)

$$
\begin{aligned}
& \mathrm{Xd}=\frac{\mathrm{GS} * \mathrm{Vw}}{1+\mathrm{e}}=\frac{2.72 * 9.81}{1+0.72}=15.513 \mathrm{KN} / \mathrm{m} 3 \\
& \mathrm{X}=\mathrm{Yd}(1+\mathrm{w}) \stackrel{15.513 *(1+0.12)=}{17.375 \mathrm{KN} / \mathrm{m} 3}
\end{aligned}
$$

b)

$$
\begin{aligned}
& \text { S.e=GS.w } \\
& S^{*} 0.72=2.72^{*} 0.12 \\
& S=45.33 \% \\
& \gamma=\frac{\mathrm{Wt}}{\mathrm{~V}}=17.375 \mathrm{KN} / \mathrm{m} 3 \\
& \mathrm{Wt} 1=17.375 \mathrm{KN} \text { if } 1 \mathrm{~m} 3 \text { volume } \\
& \mathrm{Xd}=\frac{\mathrm{Ws}}{\mathrm{~V}}=15.513 \mathrm{KN} / \mathrm{m} 3 \\
& \mathrm{Ws}=15.513 \mathrm{KN} \\
& \mathrm{Ww} 1=\mathrm{Wt}-\mathrm{Ws}=1.862 \mathrm{KN}
\end{aligned}
$$

$$
\begin{aligned}
& \text { If } \mathrm{S}=80 \% \\
& \mathrm{~S} . \mathrm{e}=\mathrm{GS} . \mathrm{w} \\
& 0.8 * 0.72=2.72 \mathrm{w} \\
& \mathrm{~W} \%=21.17 \% \\
& \mathrm{Y}=\mathrm{\gamma d}(1+\mathrm{w})=15.513 *(1+0.2117)= \\
& 18.797 \mathrm{KN} / \mathrm{m} 3 \\
& \mathrm{~V}=\frac{\mathrm{Wt}}{\mathrm{~V}}=18.797 \mathrm{KN} / \mathrm{m} 3 \\
& \mathrm{Wt} 2=18.797 \mathrm{KN} \text { if } 1 \mathrm{~m} 3 \mathrm{volume} \\
& \mathrm{Ww} 2=\mathrm{Wt} 2-\mathrm{Ws}=3.284 \mathrm{KN} \\
& \text { Weight of water in } \mathrm{KN} / \mathrm{m} 3 \text { to be } \\
& \text { added } \\
& \quad=3.284-1.862=1.422 \mathrm{KN} \text { in } 1 \mathrm{~m} 3 \\
& \text { c) } \mathrm{w}=30 \% \\
& \mathrm{S.e}=\mathrm{GS} . \mathrm{w} \\
& \mathrm{~S} * 0.72=2.72 * 0.3 \\
& \mathrm{~S}=1.133 \%>1
\end{aligned}
$$

S.e=GS.w

No.

